

Mathematical Derivations of Semianalytical Solutions
for Pumping-Induced Drawdown and Stream
Depletion in a Leaky Aquifer System

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I. Introduction

In this report, derivations of the transform-space solutions to the mathematical model describing the drawdown and stream depletion produced by a pumping well in a leaky aquifer system are presented. The back transforms of these expressions are used by *Butler et al.* [in review] to develop new insights into stream-aquifer interactions in a leaky aquifer system.

In Section II of this report, governing equations and auxiliary conditions for a leaky aquifer system hydraulically connected to a stream are presented. The corresponding equations in Laplace-Fourier space are derived in Sections III and IV using standard integral transform methods. The general transform-space solution is presented in Section V. In Sections VI and VII, simplified solutions for an unbounded domain and an unbounded homogeneous domain, respectively, are presented. The derivation of a solution for the stream-depletion rate is presented in Section VIII. The methods used to numerically invert the transform-space solutions are described briefly in Section IX.

II. Governing Equations

The problem of interest here is that of the drawdown and stream depletion produced by pumping from a fully penetrating well in the leaky aquifer system of Figure 1. Following the approach of *Butler et al.* [2001], vertical flow within the upper aquifer is neglected (Dupuit assumptions). The stream and upper aquifer are separated by a zone of relatively low hydraulic conductivity, which is represented mathematically as an incompressible layer (*Hantush* [1965]). Portions of the upper aquifer underneath the stream are confined, but can be confined or unconfined elsewhere. Flow in the aquitard is incorporated using the model of *Hantush* [1960], which includes aquitard storage but neglects lateral flow. Similar to *Hantush and Jacob* [1955], the underlying (lower) aquifer is assumed to be a unit of relatively high permeability so that heads within that aquifer are unaffected by pumping in the upper aquifer. Hydraulic properties are assumed to be a function of x , but can be linearized into a series of zones of uniform properties that are arranged parallel to the stream. Any number of zones can be considered in this derivation but three are used in *Butler et al.* [in review].

Following the approach of *Butler et al.* [2001], governing equations and auxiliary conditions can be defined for the leaky aquifer system of Fig. 1.

Aquifer Flow

$$\begin{aligned}
 & \frac{\partial^2 s(x, y, t)}{\partial x^2} + \frac{\partial^2 s(x, y, t)}{\partial y^2} - \frac{k_{sb}}{b_{sb}T(x)} s(x, y, t) (\mathcal{H}(x - x_{sl}) - \mathcal{H}(x - x_{sr})) \\
 & - \frac{k_c(x)}{T(x)} \frac{\partial s_c(x, y, z, t)}{\partial z} \Big|_{z=0} + \frac{Q}{T(x)} \delta(x - x_p) \delta(y) \\
 & = \frac{S(x)}{T(x)} \frac{\partial s(x, y, t)}{\partial t} \quad x_{lb} < x < x_{rb}, \quad -\infty < y < \infty, \quad t > 0
 \end{aligned} \tag{1}$$

Note that Eq. (1) is a condensed but generalized form of Eq. (1)-(3) in *Butler et al.* [in review].

Initial condition,

$$s(x, y, t = 0) = 0 \tag{2}$$

Left boundary condition in x can be either constant head (Dirichlet condition),

$$s(x, y, t) \Big|_{x=x_{lb}} = 0 \tag{3}$$

or no flow (Neumann condition),

$$\frac{\partial s(x, y, t)}{\partial x} \Big|_{x=x_{lb}} = 0 \tag{4}$$

Similarly, right boundary condition in x can be,

$$s(x, y, t) \Big|_{x=x_{rb}} = 0 \quad (5)$$

or

$$\frac{\partial s(x, y, t)}{\partial x} \Big|_{x=x_{rb}} = 0 \quad (6)$$

Drawdown at infinity in y is bounded,

$$s(x, y = -\infty, t) < \infty \quad (7)$$

$$s(x, y = \infty, t) < \infty \quad (8)$$

A Cauchy boundary condition could also be readily incorporated into this development. However, for large x_{lb} and x_{rb} , the solution is not sensitive to the form of the lateral boundary conditions.

Aquitard

$$\frac{\partial^2 s_c(x, y, z, t)}{\partial z^2} = \frac{S_{Sc}(x)}{k_c(x)} \frac{\partial s_c(x, y, z, t)}{\partial t} \quad x_{lb} < x < x_{rb}, \quad -\infty < y < \infty, \quad -b_c < z < 0, \quad t > 0 \quad (9)$$

Eq. (9) is equivalent to Eq. (4) in *Butler et al.* [in review].

Initial condition,

$$s_c(x, y, z, t = 0) = 0 \quad (10)$$

Continuity condition at interface of aquitard and upper aquifer,

$$s_c(x, y, z = 0, t) = s(x, y, t) \quad (11)$$

Constant head at base of aquitard,

$$s_c(x, y, z = -b_c, t) = 0 \quad (12)$$

where

x, y = Cartesian coordinates in lateral plane. The origin of the x -axis can be any arbitrary location (e.g., origin defined at right bank of stream in *Butler et al.* [in review]) and the values increase from left to right. The origin of the y axis is at the pumping well and the values increase upward, [L];

z = vertical distance from bottom of upper aquifer, [L];

t = time, [T];

$s(x, y, t)$ = drawdown in the upper aquifer, $[L]$;
 $T(x)$ = transmissivity of the upper aquifer, $[L]$;
 $S(x)$ = specific yield or storativity of the upper aquifer, $[1]$;
 k_{sb} = hydraulic conductivity of streambed, $[L/T]$;
 b_{sb} = streambed thickness, $[L]$;
 $\mathcal{H}(x - x_{sl})$ = Heaviside function ($= 0$ for $x - x_{sl} < 0$, $= 1$ for $x - x_{sl} > 0$), respectively;
 x_{sl}, x_{sr} = left and right boundary of the stream, respectively $[L]$;
 $s_c(x, y, z, t)$ = drawdown in the aquitard, $[L]$;
 $S_{Sc}(x)$ = specific storage of the aquitard, $[L^{-1}]$;
 $k_c(x)$ = hydraulic conductivity of aquitard, $[L/T]$;
 $b_c(x)$ = thickness of aquitard, $[L]$;
 x_{lb}, x_{rb} = left and right boundary of the aquifer, respectively, $[L]$;
 x_p = x coordinate of pumping well, $[L]$;
 Q = pumping rate from well located at $(x_p, 0)$, $[L^3/T]$.

A constant rate of pumping is assumed for this development. A variable rate of pumping or a cyclic pumping strategy could be readily incorporated using standard convolution approaches (*Wallace et al.* [1990])

Notation used in this report is the same as that used in the Mathematica package developed for this project and may differ from that used in *Butler et al.* [in review] because of the notation rules for Mathematica and the more generalized form of this development.

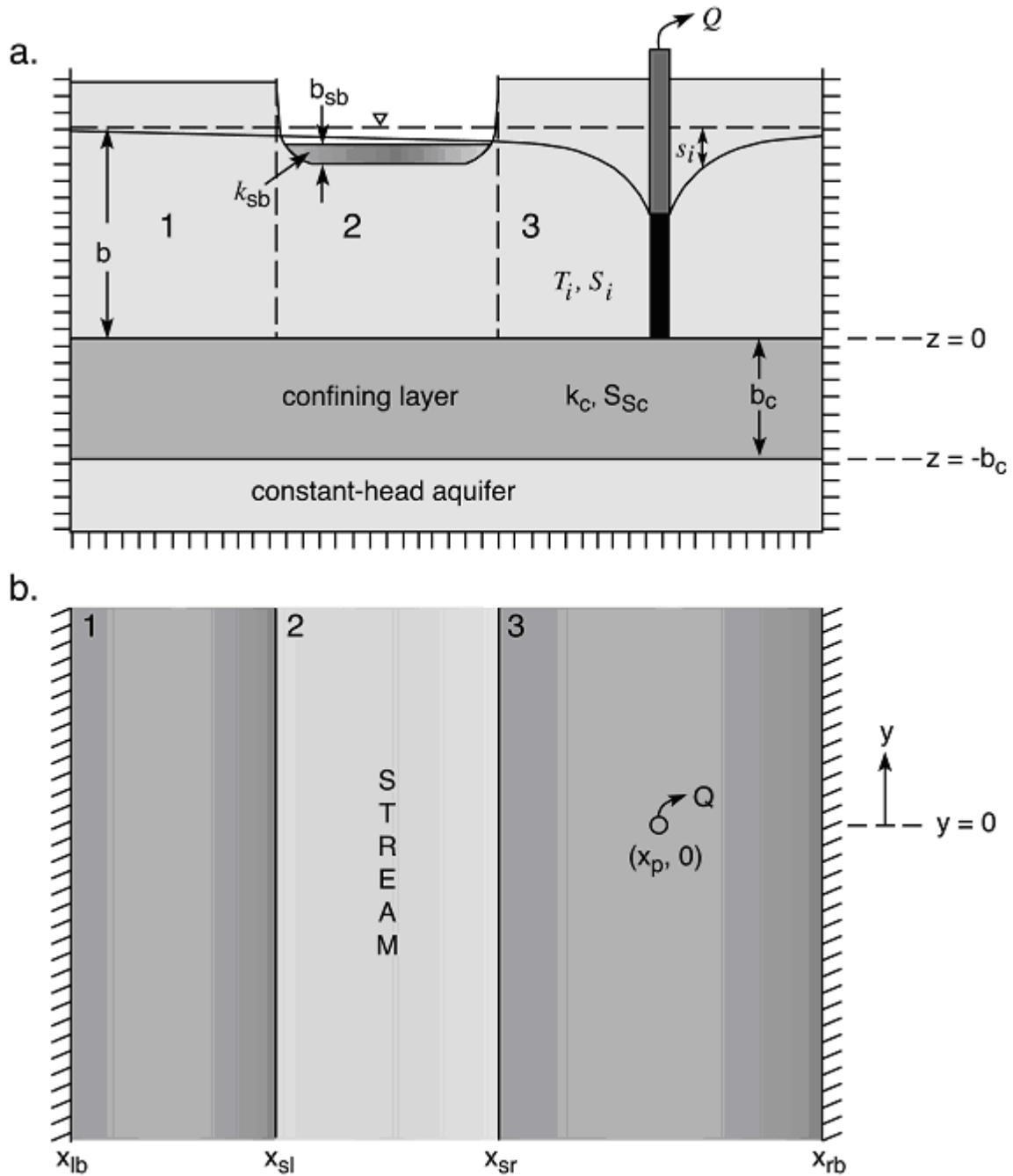


Figure 1: Schematic (a) cross-sectional and (b) areal views of the stream-aquifer system considered in this paper (notation explained in text; stream depletion in this configuration consists of vertical leakage across the low-permeability streambed).

III. Laplace Space Equations

Applying a Laplace transform in t to the equations of the previous section yields

$$\begin{aligned}
& \frac{\partial^2 \bar{s}(x, y)}{\partial x^2} + \frac{\partial^2 \bar{s}(x, y)}{\partial y^2} - \frac{k_{sb}}{b_{sb}T(x)} \bar{s}(x, y) (\mathcal{H}(x - x_{sl}) - \mathcal{H}(x - x_{sr})) \\
& - \frac{k_c(x)}{T(x)} \frac{\partial \bar{s}_c(x, y, z)}{\partial z} \Big|_{z=0} + \frac{Q}{pT(x)} \delta(x - x_p) \delta(y) \\
& = p \frac{S(x)}{T(x)} \bar{s}(x, y) \quad x_{lb} < x < x_{rb}, \quad -\infty < y < \infty
\end{aligned} \tag{13}$$

$$\bar{s}(x, y) \Big|_{x=x_{lb}} = 0 \quad \text{or} \quad \frac{\partial \bar{s}(x, y)}{\partial x} \Big|_{x=x_{lb}} = 0 \tag{14}$$

$$\bar{s}(x, y) \Big|_{x=x_{rb}} = 0 \quad \text{or} \quad \frac{\partial \bar{s}(x, y)}{\partial x} \Big|_{x=x_{rb}} = 0 \tag{15}$$

$$\bar{s}(x, y = -\infty) < \infty \quad \text{and} \quad \bar{s}(x, y = \infty) < \infty \tag{16}$$

$$\begin{aligned}
\frac{\partial^2 \bar{s}_c(x, y, z)}{\partial z^2} &= p \frac{S_{Sc}(x)}{k_c(x)} \bar{s}_c(x, y, z) \\
x_{lb} < x < x_{rb}, \quad -\infty < y < \infty, \quad -b_c < z < 0
\end{aligned} \tag{17}$$

$$\bar{s}_c(x, y, z = 0) = \bar{s}(x, y) \tag{18}$$

$$\bar{s}_c(x, y, z = -b_c) = 0 \tag{19}$$

where \bar{s} and \bar{s}_c are the Laplace transform of s and s_c , respectively, and p is the Laplace transform parameter. Note the overbar is used to indicate the dependence of s and s_c on p .

After applying the boundary conditions, the general solution for s_c at any z and its first derivative at $z = 0$ are, respectively,

$$\begin{aligned}
\bar{s}_c(x, y, z) &= \cosh \left[\sqrt{\frac{S_{Sc}(x)p}{k_c(x)}} z \right] \bar{s}(x, y) \\
&+ \coth \left[\sqrt{\frac{S_{Sc}(x)p}{k_c(x)}} \right] \sinh \left[\sqrt{\frac{S_{Sc}(x)p}{k_c(x)}} z \right] \bar{s}(x, y)
\end{aligned} \tag{20}$$

and

$$\left. \frac{\partial \bar{s}_c(x, y, z)}{\partial z} \right|_{z=0} = \sqrt{\frac{S_{Sc}(x)p}{k_c(x)}} \coth \left[\sqrt{\frac{S_{Sc}(x)p}{k_c(x)}} \right] \bar{s}(x, y) \quad (21)$$

Substituting the above equation into equation (13) produces,

$$\begin{aligned} & \frac{\partial^2 \bar{s}(x, y)}{\partial x^2} + \frac{\partial^2 \bar{s}(x, y)}{\partial y^2} - \frac{k_{sb}}{b_{sb}T(x)} [\mathcal{H}(x - x_{sl}) - \mathcal{H}(x - x_{sr})] \bar{s}(x, y) \\ & - \frac{1}{T(x)} \sqrt{\frac{S_{Sc}(x)p}{k_c(x)}} \coth \left[\sqrt{\frac{S_{Sc}(x)p}{k_c(x)}} \right] \bar{s}(x, y) + \frac{Q}{pT(x)} \delta(x - x_p) \delta(y) \\ & = p \frac{S(x)}{T(x)} \bar{s}(x, y) \quad x_{lb} < x < x_{rb}, \quad -\infty < y < \infty \end{aligned} \quad (22)$$

IV. Fourier-Laplace Space Equations

Applying a Fourier transform with respect to y to (22) and (14)-(16) produces:

$$\begin{aligned}
& \frac{d^2 \tilde{\tilde{s}}(x)}{dx^2} - \omega^2 \tilde{\tilde{s}}(x) - \frac{k_{sb}}{b_{sb} T(x)} [\mathcal{H}(x - x_{sl}) - \mathcal{H}(x - x_{sr})] \tilde{\tilde{s}}(x) \\
& - \frac{1}{T(x)} \sqrt{\frac{S_{Sc}(x)p}{k_c(x)}} \coth \left[\sqrt{\frac{S_{Sc}(x)p}{k_c(x)}} \right] \tilde{\tilde{s}}(x) + \frac{Q}{\sqrt{2\pi p T(x)}} \delta(x - x_p) \\
& = p \frac{S(x)}{T(x)} \tilde{\tilde{s}}(x) \quad x_{lb} < x < x_{rb}
\end{aligned} \tag{23}$$

$$\tilde{\tilde{s}}(x) \Big|_{x=x_{lb}} = 0 \quad \text{or} \quad \frac{d\tilde{\tilde{s}}(x)}{dx} \Big|_{x=x_{lb}} = 0 \tag{24}$$

$$\tilde{\tilde{s}}(x) \Big|_{x=x_{rb}} = 0 \quad \text{or} \quad \frac{d\tilde{\tilde{s}}(x)}{dx} \Big|_{x=x_{rb}} = 0 \tag{25}$$

where $\tilde{\tilde{s}}$ is the Fourier-Laplace transform of s , and ω is the Fourier transform variable. Note the double overbar is used to indicate the dependence of s on p and ω .

By defining,

$$\begin{aligned}
& \beta^2 = \omega^2 + \frac{k_{sb}}{b_{sb} T(x)} [\mathcal{H}(x - x_{sl}) - \mathcal{H}(x - x_{sr})] \\
& + \frac{1}{T(x)} \sqrt{\frac{S_{Sc}(x)p}{k_c(x)}} \coth \left[\sqrt{\frac{S_{Sc}(x)p}{k_c(x)}} \right] + p \frac{S(x)}{T(x)}
\end{aligned} \tag{26}$$

The above equation is simplified to,

$$\frac{d^2 \tilde{\tilde{s}}(x)}{dx^2} - \beta^2 \tilde{\tilde{s}}(x) + \frac{Q}{\sqrt{2\pi p T(x)}} \delta(x - x_p) = 0 \quad x_{lb} < x < x_{rb} \tag{27}$$

The above equation is an ordinary differential equation (ODE) that is readily solved in the following section.

V. Transform-Space Solution

Dividing the aquifer into n zones of homogeneous properties in the x direction enables Eq. (27) to be written for each zone i as

$$\frac{d^2 \tilde{s}_i(x)}{dx^2} - \beta_i^2 \tilde{s}_i(x) + \frac{Q}{\sqrt{2\pi p T(x)}} \delta(x - x_p) = 0 \quad x_i < x < x_{i+1}, \quad i = 1, \dots, n \quad (28)$$

Because $\delta(x - x_p)$ vanishes when x is not equal to x_p , the general solution for the above equation is,

$$\tilde{s}_i(x) = c_{2i-1} \sinh(\beta_i x) + c_{2i} \cosh(\beta_i x) \quad x_i < x < x_{i+1}, \quad i = 1, \dots, n \quad (29)$$

for bounded zones or

$$\tilde{s}_i(x) = c_{2i-1} e^{\beta_i x} + c_{2i} e^{-\beta_i x} \quad x_1 \rightarrow -\infty, \quad x_{n+1} \rightarrow \infty \quad (30)$$

for unbounded zones. In all cases, the constant coefficients c_i are determined from the boundary conditions.

For external lateral boundary conditions, Eq. (24) and (25) are applied to x_1 at the left boundary and to x_{n+1} at the right boundary, respectively. For inter-boundary conditions at x_i ($i = 2, \dots, n$), integrating Eq. (28) with respect to x across narrow regions containing x_i ($i = 2, \dots, n$) reveals that $\tilde{s}_i(x)$ and its first derivative are continuous at all x_i ($i = 2, \dots, n$) except at x_p ($i = i_p$) where the first derivative has a discontinuity because of the singular point at the pumping well. The boundary conditions can thus be written as,

$$\left. \begin{array}{l} \text{Dirichlet : } \tilde{s}_i(x_i) \\ \text{Neumann : } \left. \frac{d\tilde{s}_i(x)}{dx} \right|_{x=x_i} \end{array} \right\} = 0, \quad i = 1$$

$$\left(\tilde{s}_i(x_{i+1}) - \tilde{s}_{i+1}(x_{i+1}) \right) = 0, \quad i = 1, \dots, n - 1$$

$$\left. \frac{d \left(\tilde{s}_i(x) - \tilde{s}_{i+1}(x) \right)}{dx} \right|_{x=x_{i+1}} = \begin{cases} \frac{Q}{\sqrt{2\pi p T(x)}}, & i = i_p - 1 \\ 0, & \text{all other } i = 1, \dots, n - 1 \end{cases}$$

$$\left. \begin{array}{l} \text{Dirichlet : } \tilde{s}_i(x_{i+1}) \\ \text{Neumann : } \left. \frac{d\tilde{s}_i(x)}{dx} \right|_{x=x_{i+1}} \end{array} \right\} = 0, \quad i = n \quad (31)$$

Substituting Eq. (29) into Eq. (31) produces,

VII. Transform-Space Solution for Unbounded Homogeneous Aquifer

For a homogeneous domain and the zone underneath the stream ($x_{sl} < x < x_{sr}$), Eq. (26) can be simplified to

$$\beta_s^2 = \omega^2 + \frac{k_{sb}}{b_{sb}T} + \frac{1}{T} \sqrt{\frac{S_{Sc}p}{k_c}} \coth \left[\sqrt{\frac{S_{Sc}p}{k_c}} \right] + p \frac{S}{T} \quad (36)$$

Outside that zone, the equation can be further simplified to

$$\beta^2 = \omega^2 + \frac{1}{T} \sqrt{\frac{S_{Sc}p}{k_c}} \coth \left[\sqrt{\frac{S_{Sc}p}{k_c}} \right] + p \frac{S}{T} \quad (37)$$

Given the homogeneous domain, the problem can be reduced to solving a group of linear equations for four zones, the boundaries of which are defined by the pumping well and by the two sides of the stream:

$$\tilde{s}(x) = \begin{cases} \sqrt{\frac{2}{\pi}} Q \beta_s e^{-(x_p - x_{sr} + x_{sl} - x)\beta - (x_{sr} - x_{sl})\beta_s} \frac{1}{D}, & -\infty < x < x_{sl} \\ \frac{1}{\sqrt{2\pi}} Q e^{-(x_p - x_{sr})\beta - (x_{sr} + x - 2x_{sl})\beta_s} \\ (e^{2(x - x_{sl})\beta_s} (\beta + \beta_s) - (\beta - \beta_s)) \frac{1}{D}, & x_{sl} < x < x_{sr} \\ -\frac{1}{\sqrt{2\pi}} Q \frac{1}{2\beta} e^{-(x_p + x)\beta - 2x_{sr}\beta_s} \\ ((e^{2x\beta} + e^{2x_{sr}\beta})(e^{2x_{sr}\beta_s} - e^{2x_{sl}\beta_s})\beta^2 \\ 2e^{2x\beta}(e^{2x_{sr}\beta_s} + e^{2x_{sl}\beta_s})\beta\beta_s \\ (e^{2x\beta} - e^{2x_{sr}\beta})(e^{2x_{sr}\beta_s} - e^{2x_{sl}\beta_s})\beta_s^2) \frac{1}{D}, & x_{sr} < x < x_p \\ -\frac{1}{\sqrt{2\pi}} Q \frac{1}{2\beta} e^{-(x_p + x)\beta - 2x_{sr}\beta_s} \\ ((e^{2x_p\beta} + e^{2x_{sr}\beta})(e^{2x_{sr}\beta_s} - e^{2x_{sl}\beta_s})\beta^2 \\ 2e^{2x_p\beta}(e^{2x_{sr}\beta_s} + e^{2x_{sl}\beta_s})\beta\beta_s \\ (e^{2x_p\beta} - e^{2x_{sr}\beta})(e^{2x_{sr}\beta_s} - e^{2x_{sl}\beta_s})\beta_s^2) \frac{1}{D}, & x_p < x < \infty \end{cases} \quad (38)$$

in which

$$D = pT(e^{2(x_{sr} - x_{sl})\beta_s} (\beta - \beta_s)^2 - (\beta + \beta_s)^2).$$

For the case of an impermeable formation underlying the shallow aquifer of Fig. 1, the leakage (coth) term in Eq. (36) and (37) is negligible, resulting in:

$$\beta_s^2 = \omega^2 + \frac{k_{sb}}{b_{sb}T} + p \frac{S}{T} \quad (39)$$

for the zone underneath the stream ($x_{sl} < x < x_{sr}$) and

$$\beta^2 = \omega^2 + p \frac{S}{T} \tag{40}$$

for all other zones. The solution is thus the same as Eq. (38) except for the definitions of β and β_s .

VIII. Transform-Space Solution for Stream Depletion

The rate of stream depletion Δq is defined as the total volumetric discharge across the incompressible streambed at any given time (see Fig. 1 and caption). For the simplified scenario of Fig. 1 and the previous section, Δq can be expressed as:

$$\Delta q(t) = \frac{k_{sb}}{b_{sb}} \int_{-\infty}^{\infty} \int_{x_{sl}}^{x_{sr}} s_s dx dy \quad (41)$$

in which s_s is the drawdown beneath the stream.

Application of the Laplace transform to Eq. (41) and switching the x and y integrals yields:

$$\Delta \bar{q}(p) = \frac{k_{sb}}{b_{sb}} \int_{x_{sl}}^{x_{sr}} \int_{-\infty}^{\infty} \bar{s}_s dy dx = \frac{k_{sb}}{b_{sb}} \int_{x_{sl}}^{x_{sr}} \tilde{\bar{s}}_s dx \quad (42)$$

in which $\Delta \bar{q}(p)$ is the Laplace transform of Δq and $\tilde{\bar{s}}_s$ is the Fourier-Laplace transform of s_s for $\omega = 0$:

Substitution of Eq. (38) into Eq. (42) and performing the integration results in:

$$\Delta \bar{q}(p) = -\frac{1}{\sqrt{2\pi}} Q e^{-(x_p - x_{sr})\beta} (e^{x_{sr}\beta_s} - e^{x_{sl}\beta_s}) \frac{1}{D_s} \quad (43)$$

in which

$$D_s = pT((e^{x_{sr}\beta_s} + e^{x_{sl}\beta_s})\beta\beta_s + (e^{x_{sr}\beta_s} - e^{x_{sl}\beta_s})\beta_s^2).$$

IX. Numerical Inversion of Laplace-Fourier Space Solutions

The solutions in Laplace-Fourier space given in the previous sections are most readily evaluated using a numerical inversion scheme. A Mathematica Add-On package prepared by *Mallet* [2000] is used for the joint Laplace-Fourier numerical inversion. This package provides five inversion methods to invert Laplace transforms and joint Fourier/Hankel-Laplace transforms. The inversion techniques are those of *Durbin* [1974], *Stehfest* [1970], *Weeks* [1966], *Piessens* [1971], and *Crump* [1976]. The *Stehfest* [1970] algorithm, the most commonly used inversion algorithm for well-hydraulics applications, is selected for the inversion of head responses and stream depletion from Laplace space. The Fourier inversion uses the symbolic inverse Fourier transform provided in Mathematica.

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